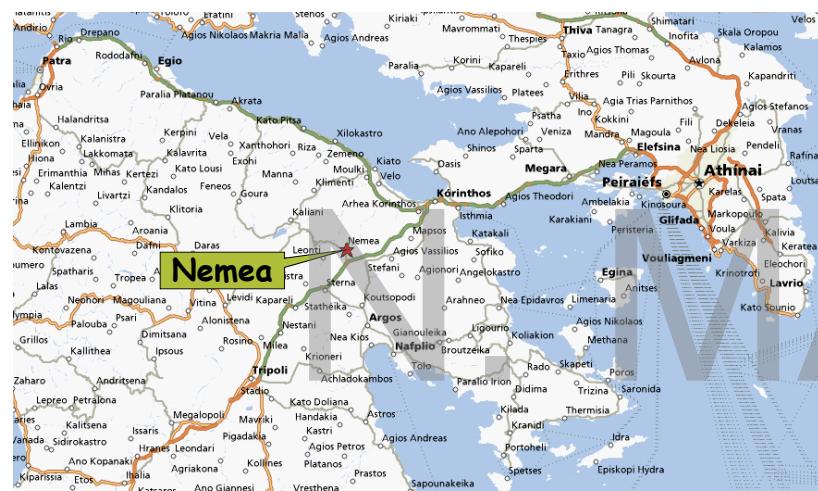


18 ΦΟΙΤΗΤΙΚΟ ΣΥΝΕΔΡΙΟ
ΕΠΙΣΚΕΥΕΣ ΚΑΤΑΣΚΕΥΩΝ 2012
14-15 ΦΕΒΡΟΥΑΡΙΟΥ 2012

Δυναμική Ευστάθεια
Αρθρωτών Κατασκευών

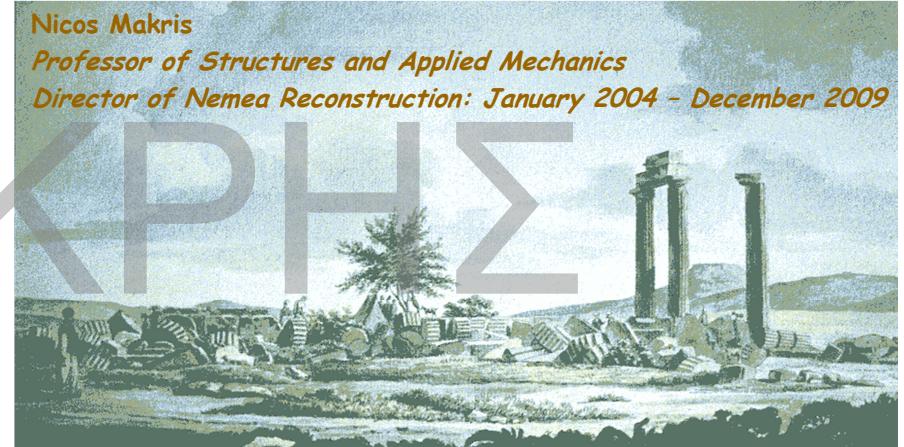
Νίκος Μακρής, Καθηγητής
email: nmakris@upatras.gr

Τομέας Κατασκευών, Πανεπιστήμιο Πατρών



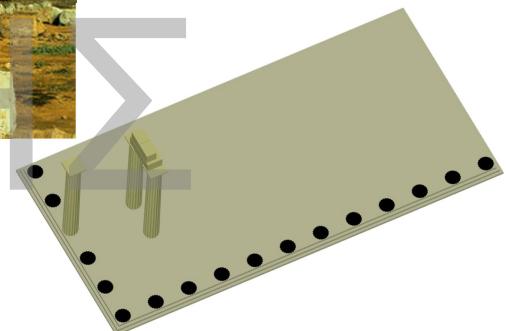
3

The Reconstruction of the North-East Corner of the
Temple of Zeus at Nemea



Nemea - Temple of Zeus, 1766

View of the three columns
standing from the ancient times



4

The challenge of reconstruction. Why reconstructing ?
Aerial View of the Temple (1977)



Columns have fallen to the outside

5

Eroded column drums



6

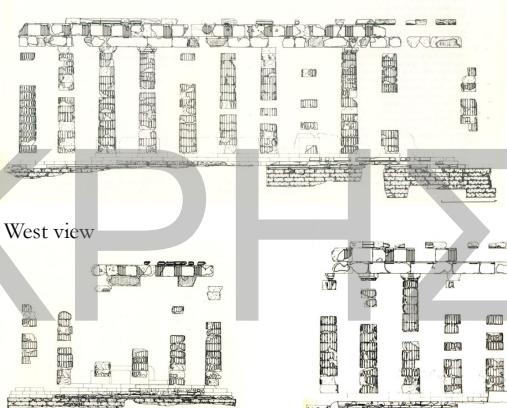
View from the west of the 3 standing columns from ancient times



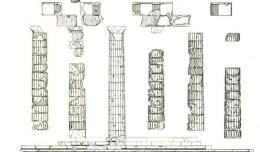
7

Synthesis of the ancient stone blocks by Frederic Cooper (1980-1982), University of Minnesota

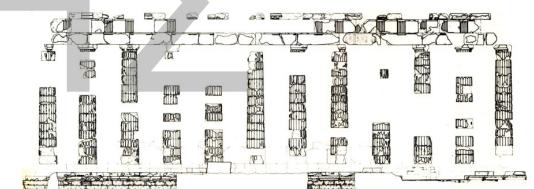
North view



East view

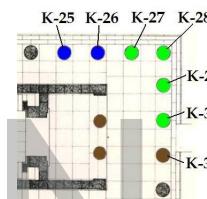
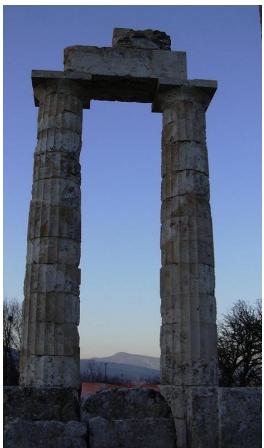


South view



8

Pilot Reconstruction: columns K-25 & K-26



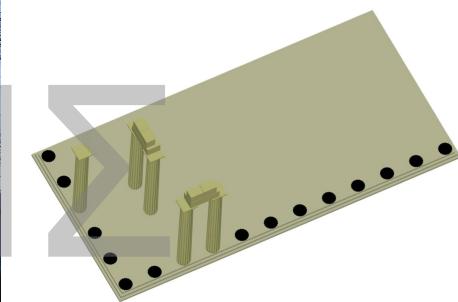
With the invaluable
contribution of:

Dr. Kostas Zambas
Prof. Manolis Korres
Mr. Ioannis Arbilias



1

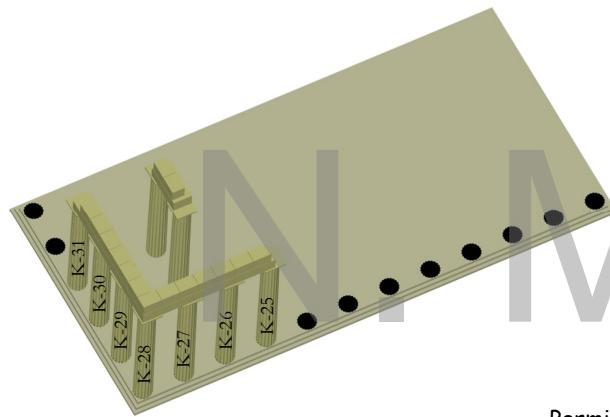
The pilot reconstruction: K-25 & K-26



Completed: Summer 2002

1

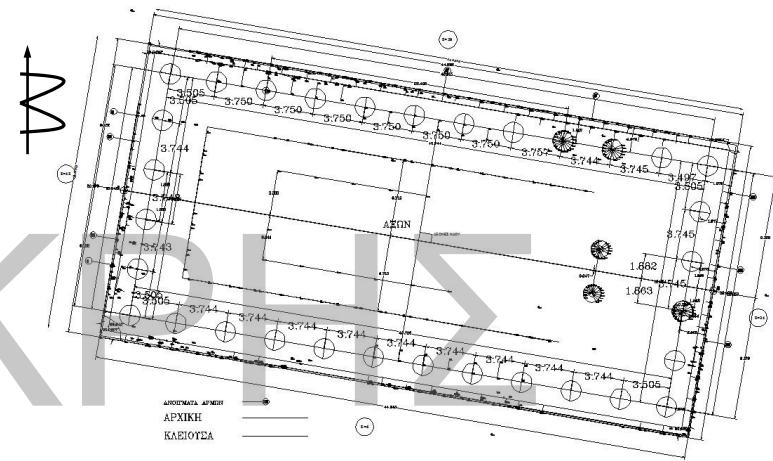
The shape of the temple after the completion of the second phase.
Reconstruction of K-27, K-28, K-29 & K-30



Permit issued on
March 04 2004

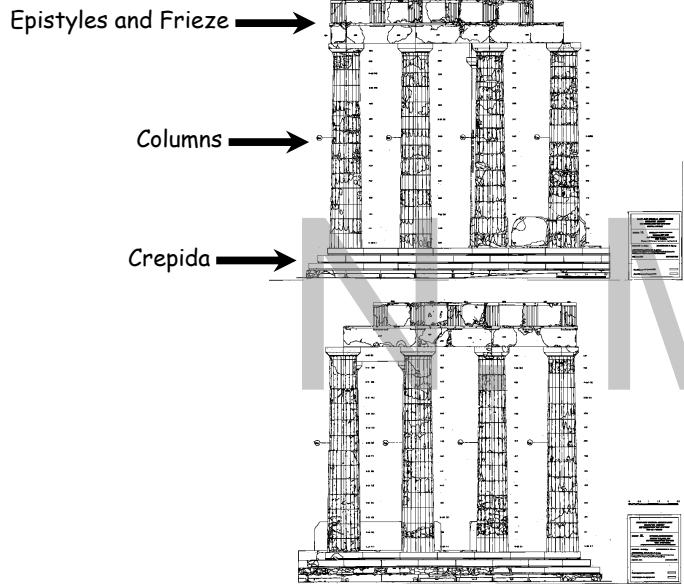
1

Detailed surveying of the current situation of the monument



Establishment of the centers of the columns based on a sophisticated optimization analysis

1



13

Reconstruction of the crepida



14

View of retrofitted ancient stones



15



16

Wire-sawing and preparation of new material



17

Retrofit of Δ-25: stylobate of ancient column K-31



18



19



20



21



22



23



24

7

Need for retrofitting the column drums



20

View of final result of retrofitted drums



2

Works on existing and new column drums



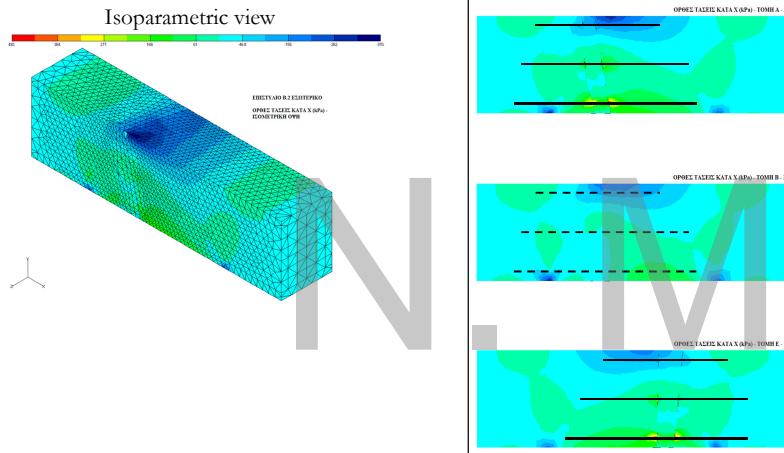
28

View of fragmented epistyles



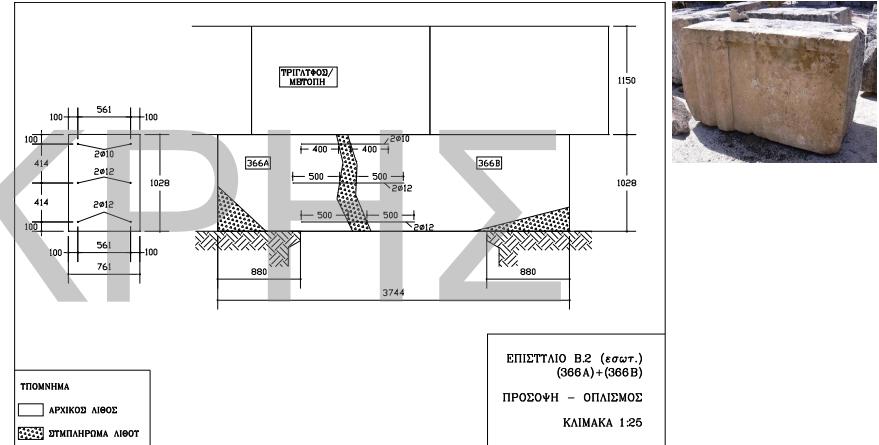
29

Normal stresses in the epistyle



3

Schematic of retrofitted epistyle



30

Les grands oeuvres de l'homme



32



33

Seismic Stability of free-standing slender structures



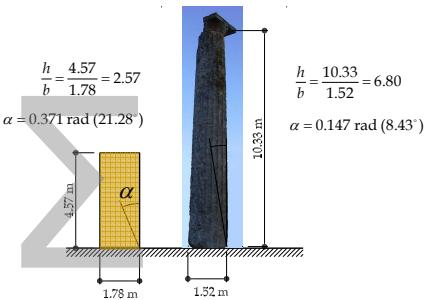
$$\frac{h}{b} = \frac{4.57}{1.78} = 2.57$$

$$\alpha = 0.371 \text{ rad (21.28°)}$$

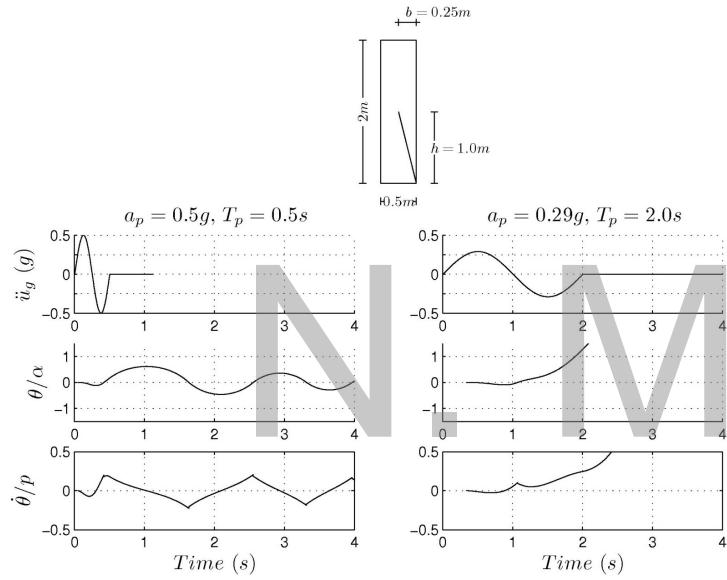


500 kV Electrical Transformer

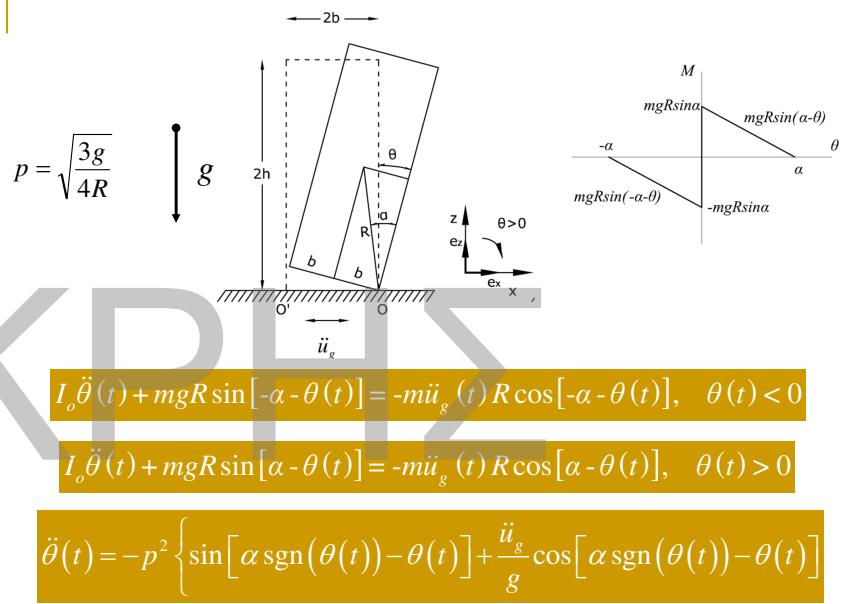
Column K-31



34



35

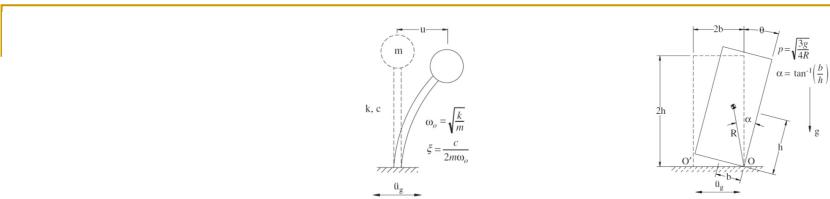


$$I_o \ddot{\theta}(t) + mgR \sin[-\alpha - \theta(t)] = -m\ddot{u}_g(t) R \cos[-\alpha - \theta(t)], \quad \theta(t) < 0$$

$$I_o \ddot{\theta}(t) + mgR \sin[\alpha - \theta(t)] = -m\ddot{u}_g(t) R \cos[\alpha - \theta(t)], \quad \theta(t) > 0$$

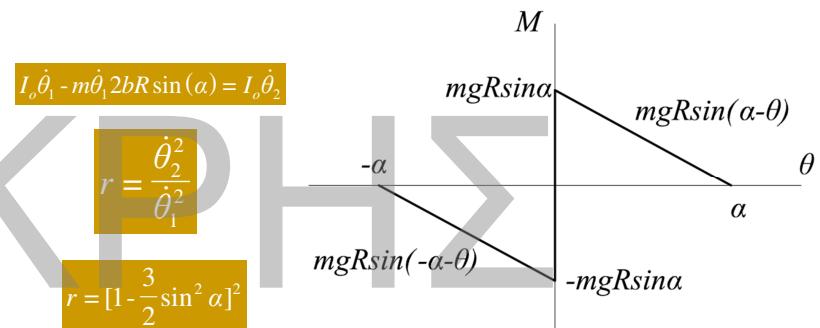
$$\ddot{\theta}(t) = -p^2 \left\{ \sin[\alpha \operatorname{sgn}(\theta(t)) - \theta(t)] + \frac{\ddot{u}_g}{g} \cos[\alpha \operatorname{sgn}(\theta(t)) - \theta(t)] \right\}$$

36



Parameters/characteristics	Damped oscillator m, c, k	Rocking rigid block b, h, g
Restoring mechanism	Elasticity of the structure	Gravity
Restoring force/moment	$F = ku$ (for linear springs)	$M = mgR \sin(\alpha - \theta)$
Stiffness at stable equilibrium	Finite	Infinite
Restoring force/moment at stable equilibrium	Zero	$mgR \sin(\alpha)$
Stiffness away from equilibrium	Positive	Negative
Frequency parameter	Undamped natural frequency: $\omega_0 = \frac{2\pi}{T_o} = \sqrt{\frac{k}{m}}$	Frequency parameter: $p = \sqrt{\frac{3g}{4R}}$ (for rectangular blocks)
Damping parameter	Viscous damping ratio: $\xi = \frac{c}{2m\omega_0}$	Slenderness: $\alpha = \tan^{-1}(b/h)$

37



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Dimensionless Products

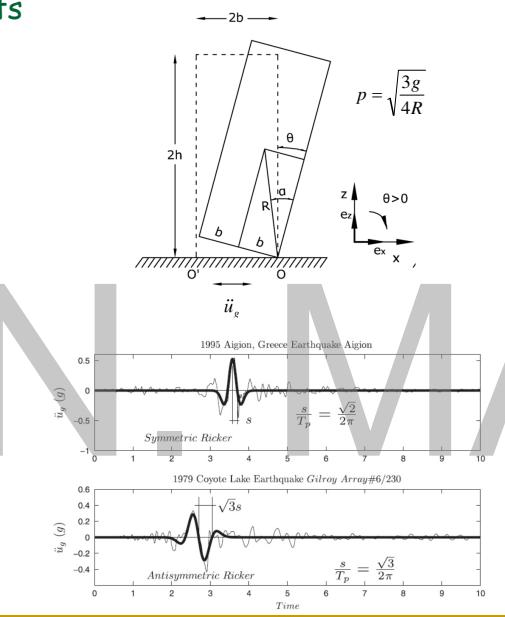
$$\Pi_\theta = \theta$$

$$\Pi_\omega = \frac{\omega_p}{p}$$

$$\Pi_\alpha = \tan(\alpha)$$

$$\Pi_g = \frac{a_p}{g}$$

$$\theta(t) = \varphi\left(\frac{\omega_p}{p}, \tan \alpha, \frac{a_p}{g}\right)$$

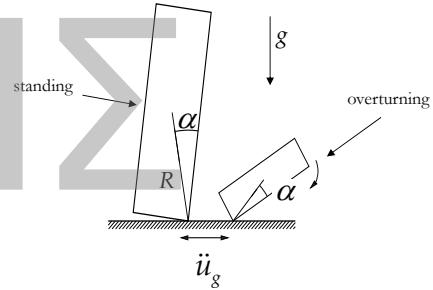


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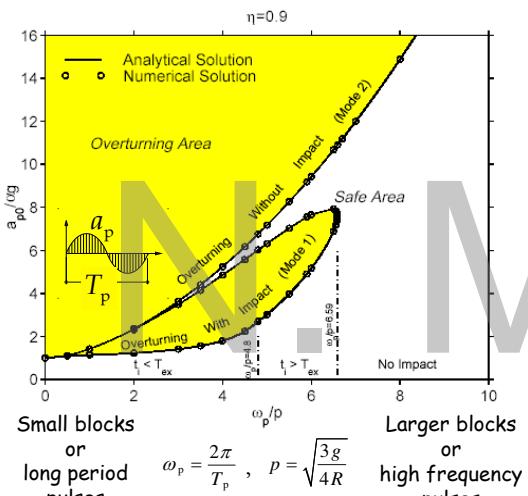
Fundamental size-frequency effect: 1963 George W. Housner

(a) The larger of two geometrically similar blocks can survive the excitation that will topple the smaller block; and (b) out of two same acceleration-amplitude pulses the one with the longer duration is more capable to induce overturning

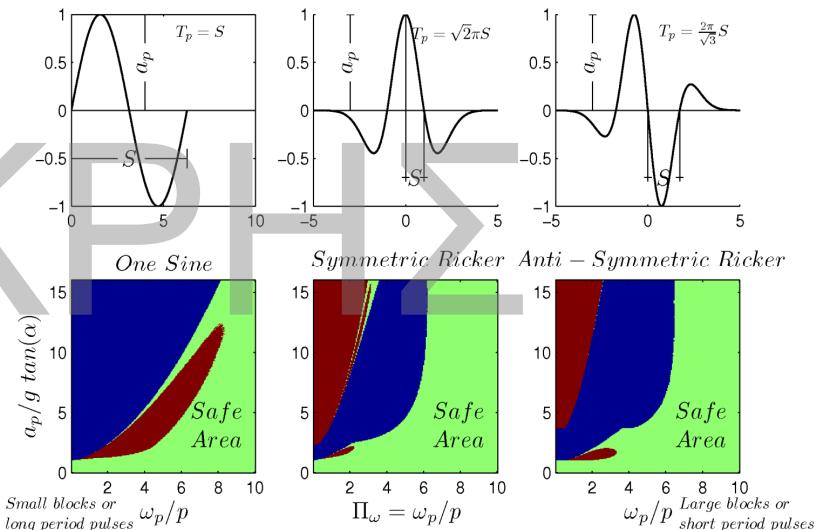
Conclusion reached from studies motivated from the destruction observed after the May 1960 earthquake in Chile.



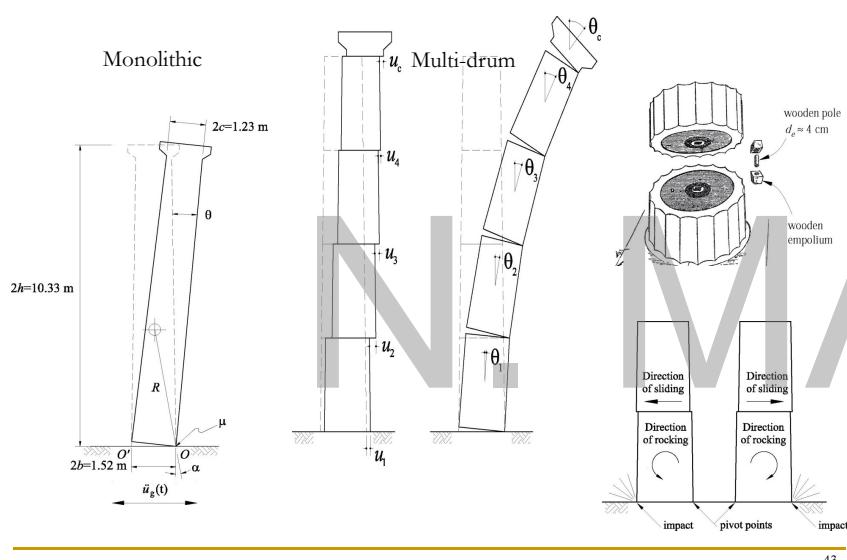
40



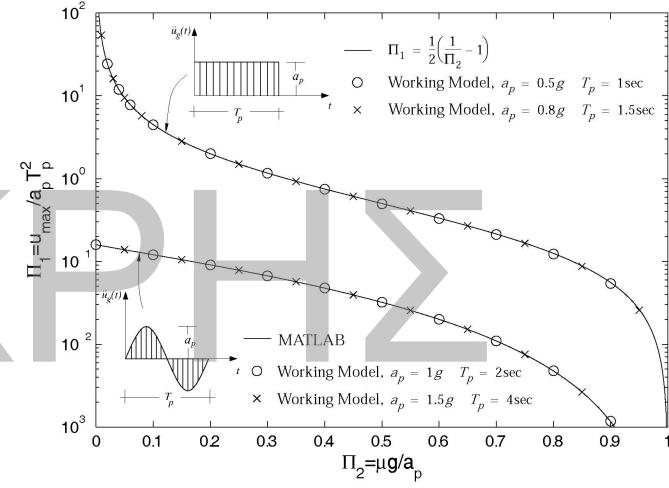
Overturning spectra of a rigid block standing free on a monolithic base

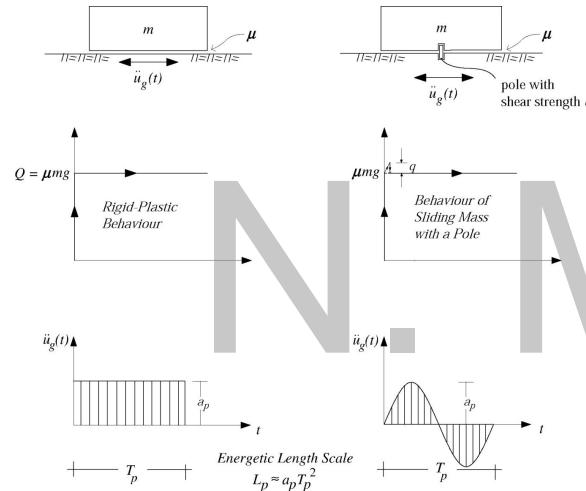


Seismic response of monolithic and multi-drum column

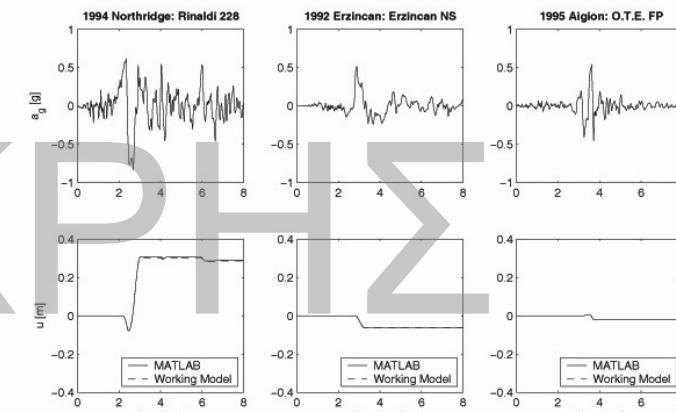


Peak Displacement of a Sliding Block Subjected to Square and One-Sine Pulse Excitations

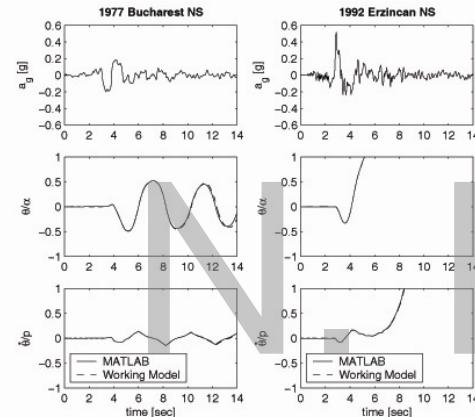




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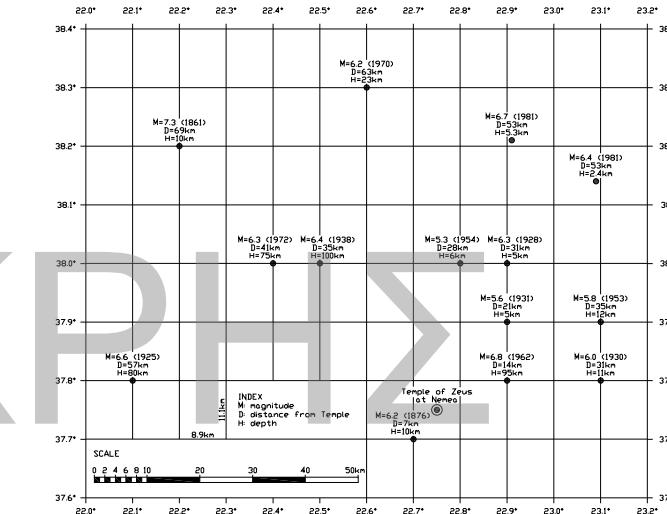


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Seismic Hazard



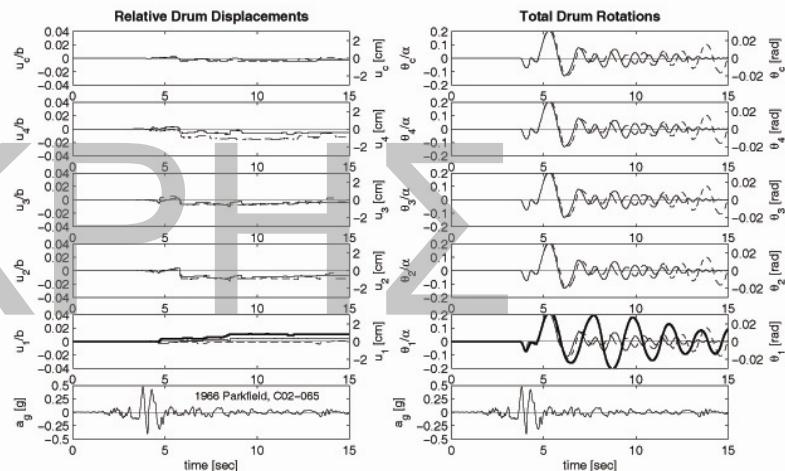
Location of selected historic earthquakes nearby the Temple of Zeus at Nemea.

48

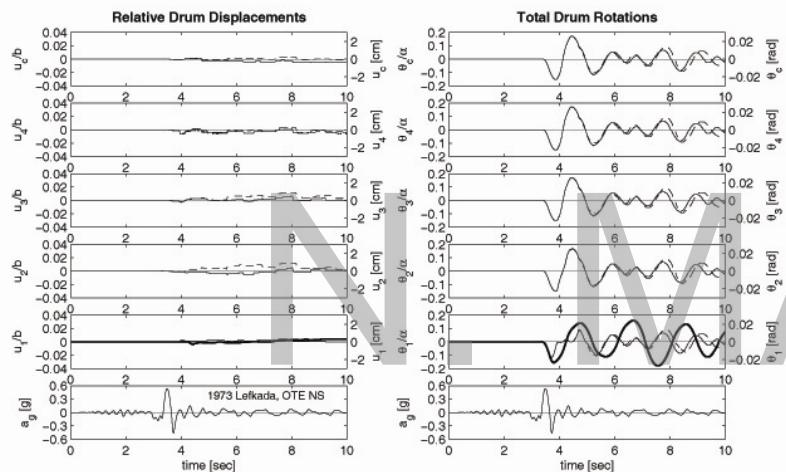
Table. Seismic Records that Represent to the Extent Possible the Seismic Hazard of the Area of the Temple of Zeus.

EARTHQUAKE	DATE	MAGN. M	RECORDING STATION	LOCAL SITE CONDITIONS	DIST. D (km)	PGA g	PGV cm/sec	PGD cm
Superstition Hills	1987 Nov. 24	6.7	El Centro Imperial Country	soil	13.9	0.36	46.4	16.0
			Westmoreland Fire Station	soil	13.3	0.21	31.0	20.1
Northridge	1994 Jan. 17	6.7	Simi Valley, Katherine Rd	soil	14.6	0.88	40.8	5.3
			Sun Valley, Roscoe Blvd	soil	12.3	0.44	38.3	10.0
Parkfield	1966 June 28	6.1	Tremblor	rock	9.9	0.36	21.4	3.5
Leukada	1973 Nov. 04	6.0	OTE building	soil	20	0.53	55.0	11.6
Aigion	1995 June 15	6.2	OTE building	soft rock	18	0.50	43.3	7.1
Kalamata	1986 Sept. 13	6.0	Nomarathia building	hard soil	5.0	0.27	24.2	5.6
Parnitha	1999 Sept. 07	5.9	Sepolia B	13 m deposits of limestone	8.0	0.32	21.5	2.7
Bucharest	1977 March 04	7.2	Bucharest	soil	160	0.20	73.4	20.4
Kocaeli	1999 Aug. 17	7.4	Sakarya	soil	3.1	0.38	79.4	71.4

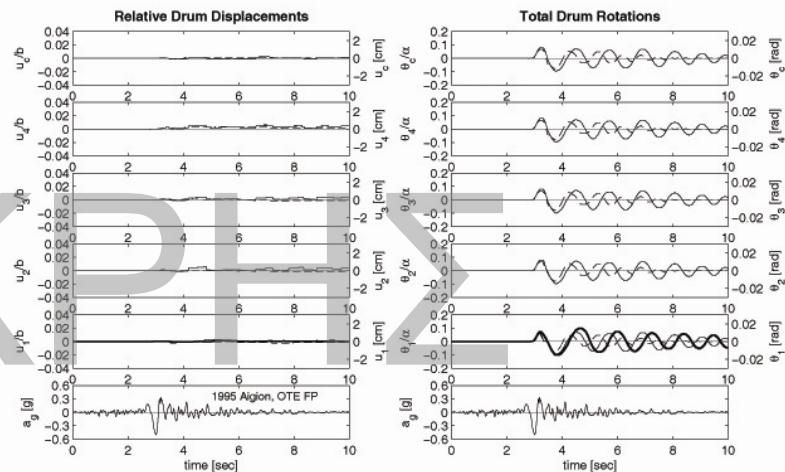
49



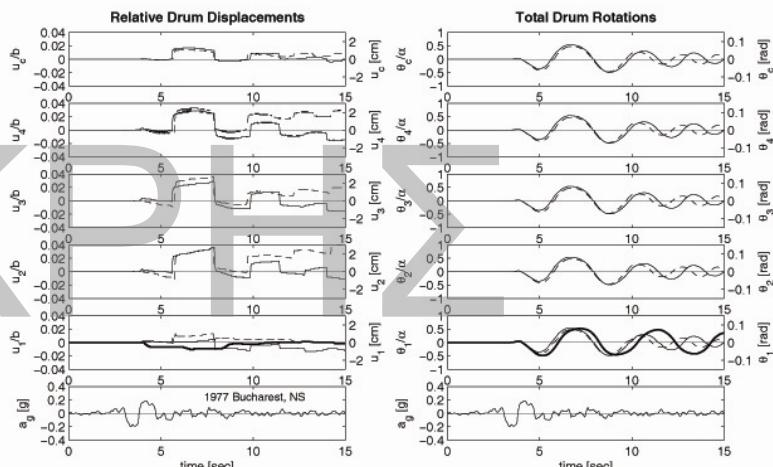
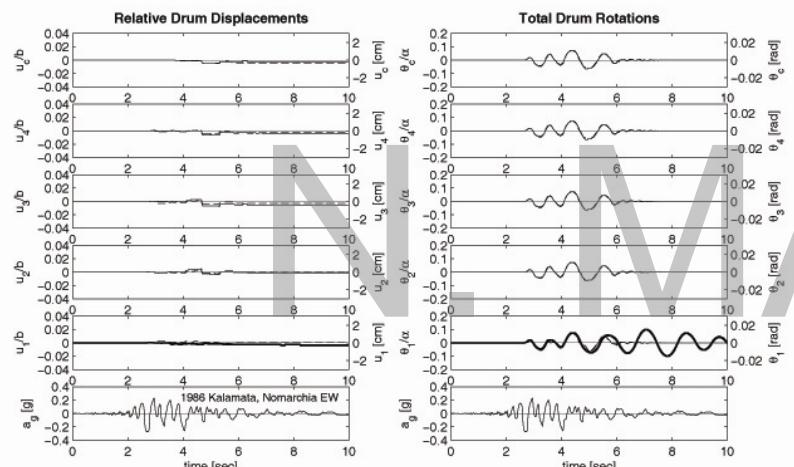
50



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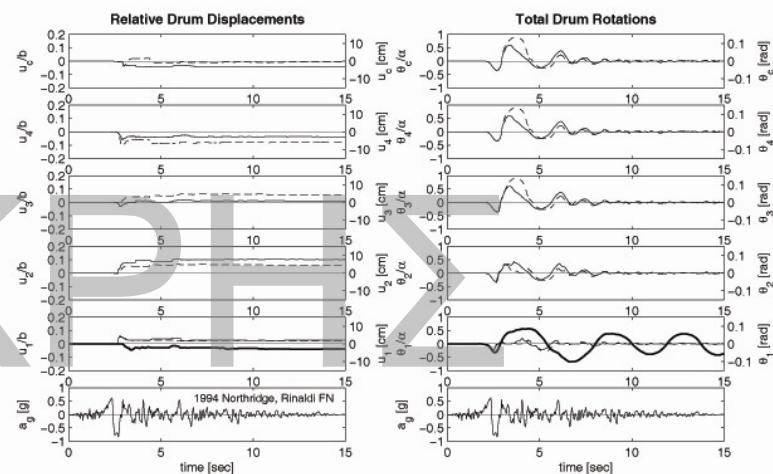
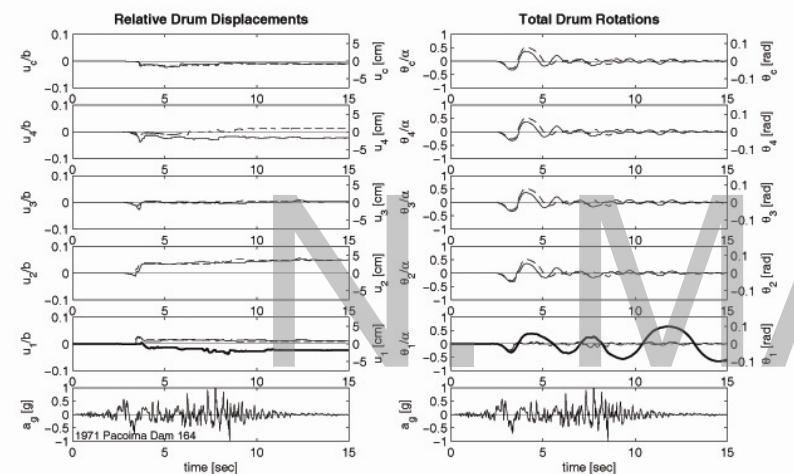


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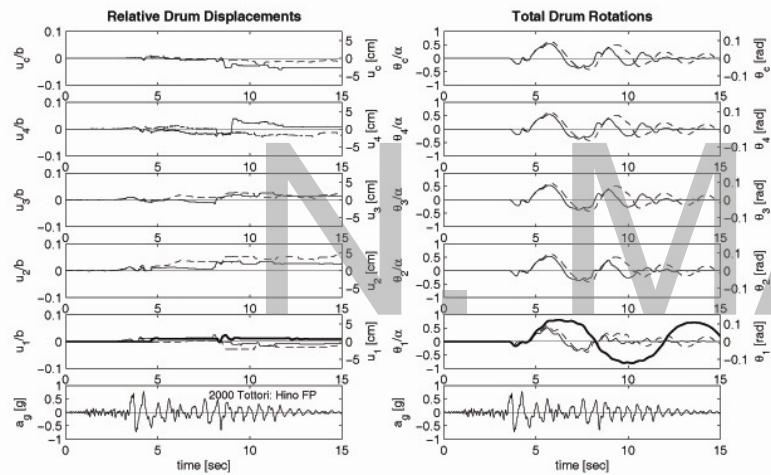
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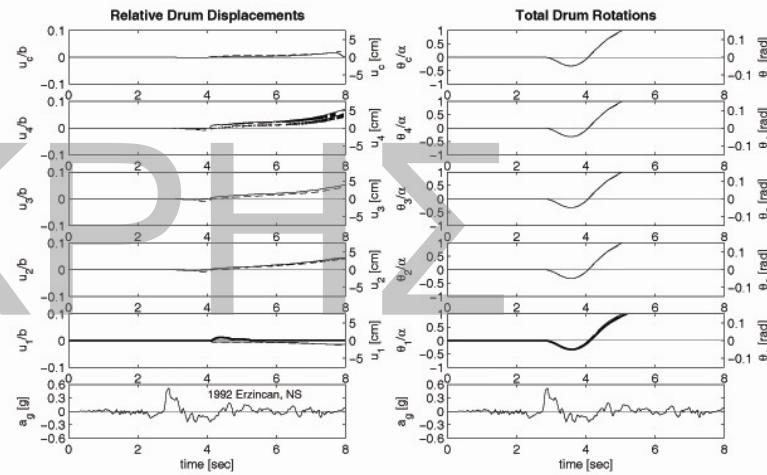


55

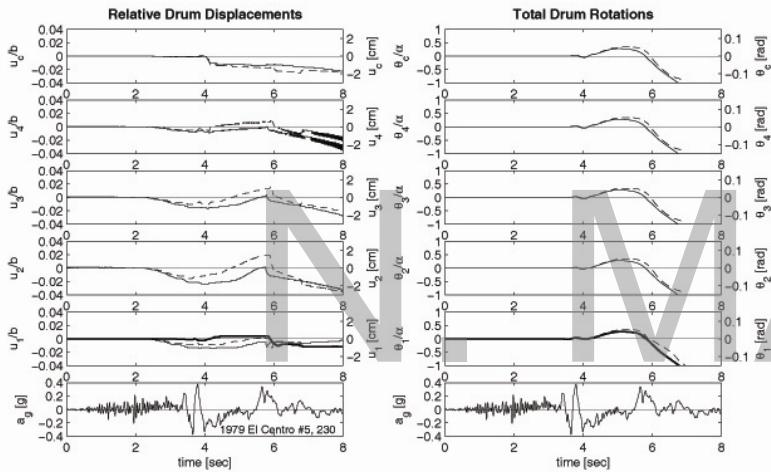
56



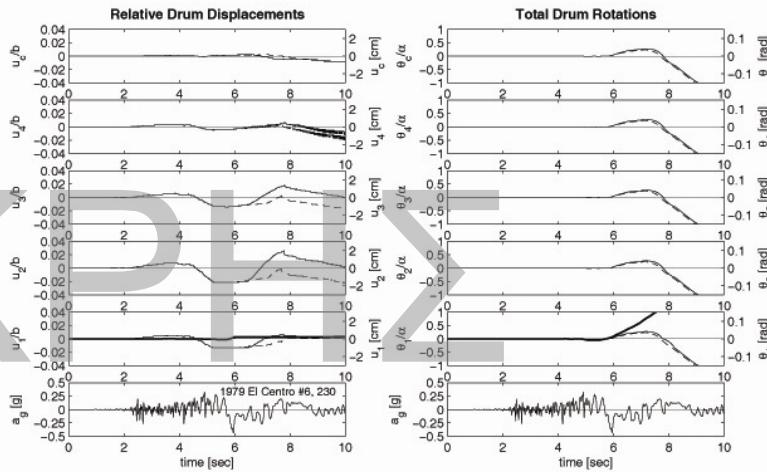
57



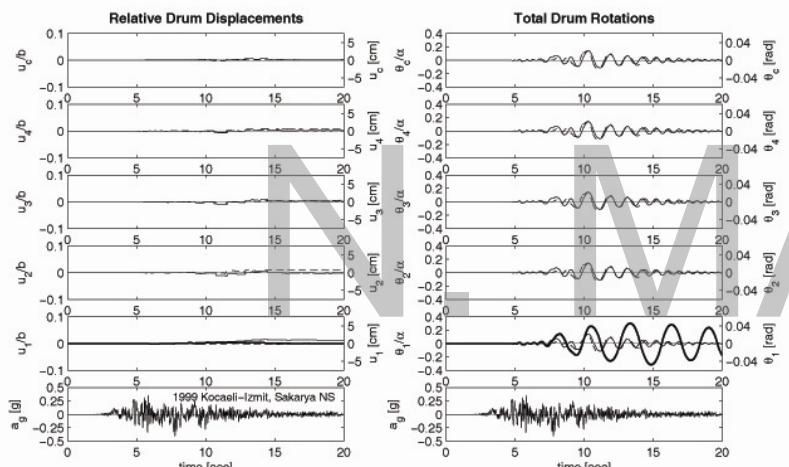
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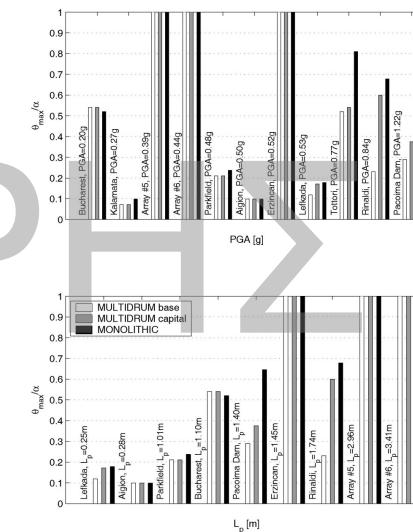


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Peak column rotations from various historic strong earthquake motions



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N. MAKPHΣ